



Preliminary Wastewater Disposal Report

Proposed Re-zoning

136 Longleat Lane Kurmond

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Sydney

Suite 603, L6, 12 Century Circuit Norwest Business Park, NSW 2153 P: 02 9659 0005 F: 02 9659 0006 sydney@barkerryanstewart.com.au

Planning

Central Coast

Suite F, 78 York Street East Gosford NSW 2250 P: 02 4325 5255 F: 02 4322 0798 coast@barkerryanstewart.com.au

Hunter

Unit 1, 17 Babilla Close Beresfield NSW 2322 P: 02 4966 8388 F: 02 4966 1399 hunter@barkerryanstewart.com.au

barkerryanstewart.com.au

Engineering

Certification

BarkerRyanStewart

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1 Introduction

This report has been prepared to detail the results of an investigation and analysis to determine the requirements of an on-site effluent disposal area for the proposed re-zoning at 136 Longleat Lane, Kurmond.

The report assesses the suitability and provides basic details of the requirements necessary to provide for an On-Site Sewerage Management (OSSM) system for the development of the site.

The OSSM system is to be designed in accordance with:

- Treat effluent on-site using a certified method in accordance with Environmental Health Protection Guidelines (DLG 1998) and AS/NZS 1547:2012 (SAI & NZS 2012).
- Comply with the requirements/objectives of the Environmental Health Protection Guidelines (DLG 1998).
- "Hawkesbury City Council Development Control Plan Part C, Section 7 Effluent Disposal" policy.

2 Site Location and Development Proposal

2.1 General

The site is located at 136 Longleat Lane, Kurmond (Lot 8 DP7565) to the East of Longleat Lane.

There is currently an existing dwelling on the site in the north western corner of the site with a shed to the south and a small dam located in the central section of the site.

A natural watercourse toward the eastern section of the site coincides with dense vegetation.



Figure 1: Aerial Photo of Site

2.2 The Locality

The site is located to the south of the main route between North Richmond and Kurrajong, towards the southern portion of Hawkesbury City Council LGA.

The areas surrounding the site are mainly developed with rural residential style development.

2.3 Infrastructure

The site is currently undeveloped with sewer provisions servicing the existing dwelling and no sewer system servicing the proposed lots.

The site is serviced by water, telecommunication and power services.

2.4 Proposed Development

The proposed development consists of four (4) lot subdivision. Proposed Lot 401 is to retain the existing dwelling, Lot 402 is to retain the existing shed while proposed Lot 403 and Lot 404 are proposed as vacant lots.

Further investigation of a suitable existing sewerage system for Lot 401 will be required to be undertaken as part of the future subdivision DA. To dispose of the effluent generated from the additional lots being Lot 402 to Lot 404, it is proposed to utilise three separate parcels of land positioned toward the west of each lot with appropriate setbacks provided from boundaries and dense vegetation. The irrigation areas will comprise:

- Lot 402 453m².
- Lot 403 453m², and;
- Lot 404 453m².

The proposed re-zoning and effluent disposal areas are shown on the plan contained in Appendix A.

2.5 Site Assessment

A preliminary site assessment revealed the characteristic soil conditions and general form of the site.

An inspection found that the generalised soil characteristic included topsoil typically 300mm thick with clayey sand at depth 300-450mm, and a clayey ironstone below this depth. These results give a soil profile indication, however, testing will need to be carried out by suitably qualified geotechnical consultant at the Subdivision DA stage.

3 Effluent Disposal Concept

3.1 Effluent Generation

To estimate the effluent generation from the proposed subdivision the following general assumptions have been made;

- 2 persons per main bedroom and 1 person for each additional room
- 150L of effluent per person per day

Assuming a future 3 main bedroom dwelling these figures equate to an effluent generation of 900 litres per day.

3.2 Site Assessment

A Site assessment rating for the on-site system has been undertaken and is shown below in Table 3.1.

Site Feature	Limitation	Rating	Comments
Flood Potential	Above 1 in 100 year flood	Minor	Effluent Disposal areas are located above the 1 in 100 year ARI.
Exposure	Exposure – High sun and wind	Minor	
Slope %	Subsurface irrigation land 10- 20%	Moderate	12-15%
Landform	Plains and side slopes	Minor - Moderate	
Run On and U/S Seepage	Non to Low	Minor to Moderate	
Erosion Potential	Sign of erosion	Moderate	The proposed irrigation area will be set away from the erosion prone areas.
Site Drainage	No visible sign of surface dampness	Minor	
Fill	None	Minor	
Buffer Distance	Refer section 3.3	Minor	
Land Area	Refer section 3.3	Minor	
Rock Outcrops	None	Minor	

A detailed soil assessment rating for on-site systems will be required to be undertaken at the Subdivision DA stage. However, based on preliminary site observations it is believed that an appropriate on-site disposal system for each lot will be possible for the proposed re-zoning layout.

136 Longleat Lane, Kurmond

Based on a preliminary assessment and our experience the site is considered to be suitable for onsite disposal of the effluent generated by future dwellings within the recommended disposal envelope as indicated in Appendix A. However a detailed soil assessment will be required to be undertaken at the subdivision DA stage to confirm the exact sizing and location of the effluent disposal envelopes.

3.3 Disposal System

The following parameters have been used in the calculations to determine the required disposal area:

- Concentration of total Nitrogen in effluent 10mg/L
- Critical Loading rate of Nitrogen for pasture 20mg/m²/day
- Concentration of Total Phosphorous in effluent 5mg/L
- Critical Loading rate for Phosphorous 5mg/m²/day
- Phosphorous Sorption Capacity between 6,290kg/ha and 8,738kg/ha (Average of 7,514kg/ha)
- The 50th percentile rainfall figures and pan evaporation figures have been obtained from the Bureau of Meteorology's website for the station at the Richmond RAAF Base. They follow in Table 3.2 below.
- Evapotranspiration is the pan evaporation by a 0.7 crop factor
- Representative soil permeability 0.06m/d
- Design Peculation Rate 5mm/week

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Temperature												
Mean maximum temperature (°C)	30.0	29.1	26.8	23.9	20.6	17.9	17.4	19.7	22.7	25.0	26.7	28.5
Mean minimum temperature (°C)	17.6	17.7	15.6	11.4	7.6	4.9	3.6	4.4	8.0	11.0	14.1	16.0
Rainfall												
Mean rainfall (mm)	78.5	125.8	74.2	48.9	52.4	48.0	31.2	30.7	49.7	52.8	83.5	61.6
50 th Per Rain (mm/m)	67.4	141.2	70.2	40.0	37.1	35.2	24.6	16.4	33.2	32.8	82.2	55.4
Mean number of days of rain ≥ 1 mm	7.6	8.5	7.6	5.8	5.7	5.4	4.2	3.4	4.7	5.9	8.1	6.5
					Evapo	ration						
Mean evaporation (mm)	182.9	137.2	124.0	62.0	68.2	51.0	58.9	83.7	114.0	142.6	147.0	173.6

Table 3.2 – Meteorological Data

Table 3.3 – Calculations

Summary of Disposal Area Calculations							
Hydraulic Loading (Equivalent 3 Bedrooms)	900L						
Areas							
Minimum Area Method 453m ²							

The On-Site Sewerage Management System proposed at the subdivision DA stage will need to comply with the items outlined above.

It should also be noted that the Environment and Health Guidelines (1998) states that "soils with a phosphorus sorption capacity of over 6000 kilograms per hectare (calculated to a depth of 1 metre) should not be limiting for irrigation areas." Geotechnical testing will be required to confirm soil absorption rates within in those stated in the guidelines.

Preliminary calculations carried out indicate an area of 453m² required for effluent disposal for each lot.

The various calculations were undertaken in accordance with the Environment and Health Guidelines (1998) and other relevant industry standards. Details of the calculations are included in Appendix E.

3.4 Available Effluent Disposal Area and Buffer Distances

The available disposal area for each of the proposed lots has been calculated as:

- Lot 402 453m².
- Lot 403 453m², and;
- Lot $404 453 \text{m}^2$.

Taking setbacks and other site constraints into consideration, areas in excess of 500m² for each proposed lot are available. These areas are shown in Appendix A.

The buffer distances provided around the effluent disposal area (for a subsurface irrigation system) are listed below together with the provided distances.

- Permanent waters –100m (recommended 100m)
- Domestic groundwater well 220 260m (TBC) (recommended 250m)
- Intermittent waterways 40m (recommended 40m)
- Upstream and side boundaries min 3m (recommended 3m)
- Downstream boundaries min 6m (recommended 6m)

The location of the two bore holes identified will need to be confirmed. It is noted that the northern borehole is located upstream of proposed effluent treatment area while the western borehole is located beyond the crest of the hill to the west of the site.



Groundwater Bores

Figure 2: NSW Natural Resource Atlas

Clarification of the distances of boreholes from the effluent disposal areas will be required during further investigation and design of effluent disposal areas at the subdivision DA stage. This preliminary assessment suggests that the effluent disposal areas should be possible without negative impact subject to further investigation and design.

4 Conclusion

From the preliminary assessment for on-site effluent disposal for the proposed re-zoning, each lot is considered suitable to the effective dispose of effluent provided:

- The disposal area required for each proposed lot will be approximately 453m² while a provision of over 500m² has been allowed for.
- Buffers and setbacks have been checked and maintained generally in accordance with aforementioned on-site effluent disposal policies.
- Buffer distance to existing boreholes will need to be confirmed with the necessary measures taken during design to ensure necessary precautions are taken should the buffer distance fall short of the requirement.

The clarifications noted above will be required to be confirmed as part of a more detailed investigation at the Subdivision DA stage and can be conditioned for as part of any development application consent issued.

Appendix A

Proposed Re-zoning Layout and Effluent Disposal Areas



Appendix B

Calculations

Design Effluent Flow	(Q)	l/day	900												
Design Perculation Rate	(R)	mm/wk	5												
Parameter	Symbol	Formula	Units	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Days in Month	(D)		days	31	28	31	30	31	30	31	31	30	31	30	31
Precipitation	(P)	50th Percentile	mm/month	67.4	141.2	70.2	40	37.1	35.2	24.6	16.4	33.2	32.8	82.2	55.4
Evaporation	(E)		mm/month	182.9	137.2	124.0	62.0	68.2	51.0	58.9	83.7	114.0	142.6	147.0	173.6
Crop Factor	(C)			0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7	0.7
Outputs															
Evaprtanspiration	(ET)	(E x C)	mm/month	128.0	96.0	86.8	43.4	47.7	35.7	41.2	58.6	79.8	99.8	102.9	121.5
Percolation	(B)	(R / 7) x D	mm/month	22.1	20.0	22.1	21.4	22.1	21.4	22.1	22.1	21.4	22.1	21.4	22.1
Outputs		(ET + B)	mm/month	150.2	116.0	108.9	64.8	69.9	57.1	63.4	80.7	101.2	122.0	124.3	143.7
Inputs															
Retained Precipitation		P*0.75	mm/month	50.6	105.9	52.7	30.0	27.8	26.4	18.5	12.3	24.9	24.6	61.7	41.6
Possible Effluent Irrigation	(W)	(ET+B)-P	mm/month	99.6	10.1	56.3	34.8	42.1	30.7	44.9	68.4	76.3	97.4	62.7	102.1
Actual Effluent Production	(1)	H/12	mm/month	60.5	60.5	60.5	60.5	60.5	60.5	60.5	60.5	60.5	60.5	60.5	60.5
Inputs		(P + I)	mm/month	111.0	166.4	113.1	90.5	88.3	86.9	78.9	72.8	85.4	85.1	122.1	102.0
		(P+I)-(ET+B)	mm/month	-39.2	50.3	4.2	25.6	18.4	29.7	15.5	-8.0	-15.9	-36.9	-2.2	-41.7
	(S)		minymonth			54.5	80.1	98.5	128.2	143.8	135.8	119.9	83.0	80.8	39.2
Storage Cumulative Storage	(S) (M)	(, ()	mm	0.0	50.3	54.5			-						
Storage	(S) (M)	(*) (= : =)	mm	0.0	50.3	54.5	0011								
Storage Cumulative Storage		365 x Q /H	mm m ²	0.0 453	50.3	54.5	0011								
Storage	(M)				50.3	34.3	0012								